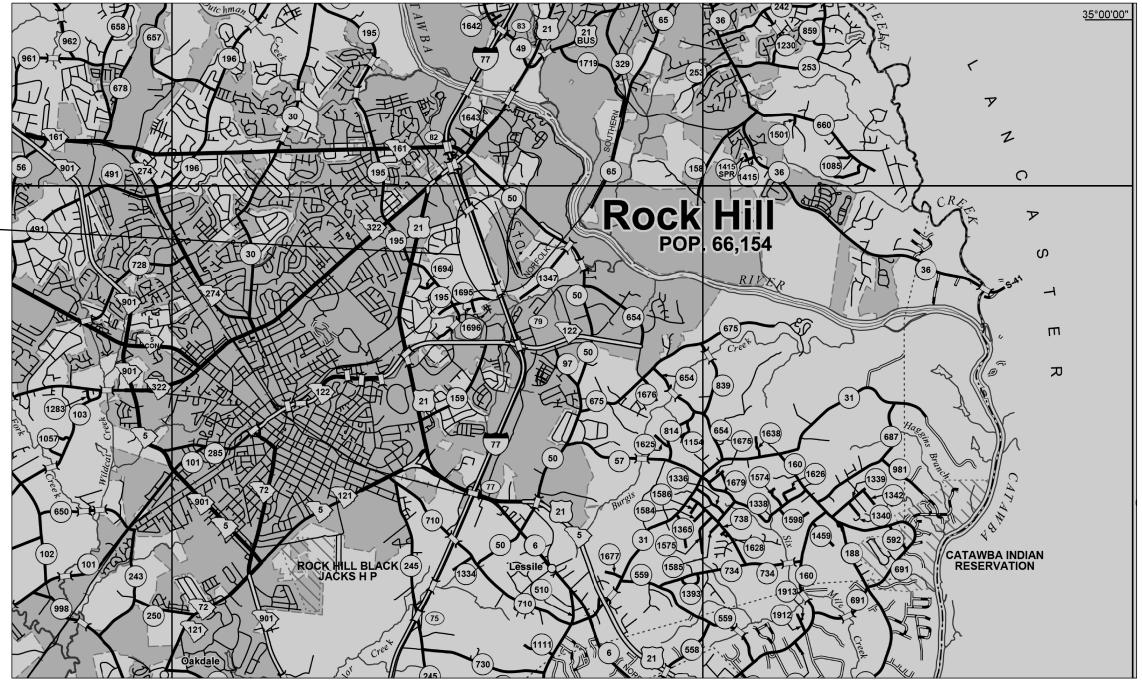
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	INDEX OF SH	IEETS	
	 Title Sh Genera Bridge F Typical Interior MSE W 	I Notes Plan and Profile Section Bent Elevation	
	0. IVISE VV	ans	
			ROA
		SIT	
	TRAFFIC DATA		
-	<u>2023</u> ADT <u>13,800</u> V.P.		
	<u>2043</u> ADT <u>30,900</u> V.P. TRUCKS <u>2</u> %	.D.	





YORK COUNTY PROJECT ID P038652 -77 PANTHER INTERCHANGE -AD S-1923 (ONE CAROLINA DRIV DNSTRUCT OVERPASS OVER 1-72



LAYOUT

]
0.607	MILES
0.054	MILES
0.680	MILES
0.000	MILES
3.849	MILES
	0.054 0.680 0.000

NOTE: EXCEPT AS MAY OTHERWISE BE SPECIFIED ON THE PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIALS AND WORKMANSHIP ON THIS PROJECT SHALL CONFORM TO THE SOUTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION (2007 EDITION) AND THE STANDARD DRAWINGS FOR ROAD CONSTRUCTION IN EFFECT AT THE TIME OF LETTING.

		PIN	SHEET
		P038652-B01	1
VE) 7	Approximate Location of Brit Latitude 34°-57 - 33 Longitude 20°-58°-45	P038652-B01	NO.
PLANS PREPARED BY:	ENGINEER OF RECORD	١S	
	FOR CONSTRUCTION :	DATE	

MATERIAL & WORKMANSHIP

Provide all material and workmanship in accordance with the South Carolina Department of Transportation 2007 Standard Specifications for Highway Construction, unless otherwise specified on the Plans or in the Special Provisions.

COORDINATION OF PLANS, SPECIFICATIONS, AND SPECIAL PROVISIONS

Generally, in case of discrepancy, this General Notes sheet governs over the Standard Specifications but the remainder of the plans govern over notes on this sheet and Special Provisions govern over all. See Subsection 105.4 of the Standard Specifications.

WATER ELEVATIONS

The water elevations shown in the plans are for information only and the actual water elevation during construction may vary depending on weather conditions and seasonal fluctuations.

COMPLETION DATES

On inside face of right side barrier parapet/railing at beginning of bridge and on left side barrier parapet/railing at end of bridge, place year of completion adjacent to guardrail attachment. Place this completion date so that it will not be covered by the guardrail connector when it is installed. Recess numbers in the concrete using numbers fabricated from reusable/durable material that is approved by the RCE. Provide numbers in accordance with SCDOT Standard Drawing No. 702-305-00.

REINFORCING STEEL

Fabricate reinforcing bars in accordance with the current C.R.S.I. Manual of Standard Practice except for ties, stirrups, and welded hoops.

Provide all ties and stirrups with 135° hooks that have extensions no less than the larger of ten bar diameters or six inches. This 135° hook requirement does not apply to stirrups extending from prestressed concrete beams.

The fabrication tolerance for out-to-out dimension of welded hoop diameter is $\pm \frac{1}{2}$ inch.

Do not use lap splices in column and shaft reinforcing steel.

PRESTRESSED CONCRETE BEAMS

Beam lengths given are based on horizontal span only. Increase lengths to correct for concrete shrinkage, concrete shortening when the strands are cut, and for beams being on a grade.

All overhang brackets in the top flange of exterior beams shall be galvanized in accordance with AASHTO M 111, AASHTO M 232, or ASTM F 2329 as appropriate and shall be detailed accordingly in the shop plans.

CONCRETE

Provide the class of concrete as noted in the contract documents. For cast-in-place structural elements, use Class 4000 concrete where the class of concrete is not specified in the contract documents.

When holes are cast in beams to accommodate falsework, fill the holes with a non-shrink structural grout suitable for overhead repairs after falsework is removed.

After erection of the beams and prior to the erection of the deck slab falsework, measure beam cambers. Compare the measured beam cambers to the values shown on the Plans to aid in determining if field adjustments are needed. Submit beam camber measurements and any proposed field adjustments to the RCE for approval. All cost of performing this work is considered incidental to the Contract and no additional compensation is allowed for the performance of this work.

Payment for concrete in slab is based on theoretical plan quantity. No adjustment is made for variation in camber.

Chamfer all exposed edges $\frac{3}{4}$ unless otherwise noted.

The minimum acceptable concrete cover for reinforcing steel is $\frac{1}{2}$ less than the plan dimensions when required by reinforcing bar fabrication tolerances.

Cast build-ups and shear keys on bent caps monolithic with the cap unless indicated otherwise in these plans, Construct the top of each build-up level.

GRINDING & TEXTURING CONCRETE DECKS

For bridge stage construction projects, grind and texture the bridge decks as necessary near the stage longitudinal construction joints in order to meet the longitudinal and transverse rideability and rolling straightedge requirements of the Contract.

Prior to casting any closure pour, grinding, or texturing, make profile line surveys (2 to 6 as determined by the RCE) of each stage of the bridge decks. Make one of these profile line surveys for each stage along the edge of the deck adjacent to the closure pour. Compare the surveys within each stage and compare the surveys of each stage to surveys of the adjacent stage to aid in determining the amount of grinding and texturing needed to meet the rideability and rolling straightedge requirements. Submit all grinding and texturing procedures, plotted survey profiles, and proposed grinding depths to the RCE for approval. Maintain a final cover of 2"minimum over the bridge deck reinforcing steel.

Follow the above procedures for all stages of the work. For all surveys performed on the same bridge, use identical stations for survey shots in order to facilitate survey comparisons. All costs for performing, evaluating, and submitting the surveys are considered incidental to the Contract and no additional compensation is allowed for the performance of this work.

Payment for grinding and texturing concrete bridge decks at the junction of new and existing bridge deck slabs is determined in accordance with Subsection 702.6 of the Standard Specifications. No payment is made for grinding and texturing of new bridge decks to correct irregularities and excessive deviations.

In setting forms for structural steel or prestressed concrete beam spans, apply an allowance to the design finished grade to compensate for computed dead load deflections.

Prior to making deck pours on any stage construction work, and bridge widening projects, consider and make adjustments as necessary for partially loaded beams adjacent to closure pour areas. Verify that any proposed adjustment on partially loaded beams does not create a change in the deck thickness or a reduction in the concrete cover over the reinforcing steel. Welded studs on steel beams and reinforcing steel extending up out of prestressed beams shall meet the requirements for a composite section (extend up into the deck past the bottom mat of reinforcing steel) regardless of any adjustments.

PERMANENT STEEL BRIDGE DECK FORMS

Notify the Department and the Fabricator of the beams if using this option so that shop plans can be properly detailed.

DRIVEN PILE FOUNDATIONS

Where prestressed concrete piles are to be driven through fill, install piles in pre-bored holes extending to the original ground. For square prestressed concrete piles, bore holes having a minimum diameter of 1.25 times the nominal pile size. Include all cost of pre-boring fills for pile installation in the unit price bid for the piles.

EXCAVATION FOR END BENTS

If a concrete footing is used for the end bent, the excavation below that included for the cap and berm in the above paragraph is paid for at the unit price bid for excavation. Include excavation above this in the unit price bid for class of concrete specified in the Plans.

ALLOWANCE FOR DEAD LOAD DEFLECTION & SETTLEMENT

Permanent stay-in-place steel bridge deck forms for concrete deck slabs may be used at the Contractor's option.

Where piles occur in fill, place fill before driving piles.

Include all cost of excavation necessary to construct end bents and to remove material under superstructure to an elevation twelve inches below tops of end bent caps, in the unit price bid for class of concrete specified in the Plans.

STRUCTURAL STEEL

Layout dimensions and standard lengths of beams shown dimensions which must be increased when bridge is on

When holes are placed in webs to accommodate falsework strength bolts in the holes after falsework is removed

Notify the Department of the name and address of the structural steel as soon as the Fabricator has been g fabricate so that the inspection procedure can be set

Do not field or shop weld erection hardware to the st

Make all bolted connections with $\frac{7}{8}''$ dia. ASTM A 325 be otherwise indicated.

Generally, holes for $\binom{7}{8}''$ dia. bolts shall be $\binom{10}{16}''$ dia. straight girder spans, oversized holes, 3/16" larger t be used in diaphragms and/or crossframes and their cor plates provided hardened washers are installed over o in the outer ply of the material gripped.Hardened was under DTIs on oversized holes. In every case install washer under the element turned for each bolt of a bo Indicate on the Shop Plans which holes are to be over hardened washers are required. No additionalpayment costs associated with the use of oversize holes and f hardened washers as necessary.

PAINT FOR STRUCTURAL STEEL

Paint structural steel in accordance with Section 710 Specifications.

BEARING ASSEMBLIES

If bearing assemblies support weathering steel beams bearing assembly components from weathering steel and NS2 Paint System, Galvanize all other bearing assembl with AASHTO M 111, AASHTO M 232, or ASTM F 2329 as app

After the required field welding of painted bearing as repair the weld areas and/or any damaged areas to the with Subsection 710.4.2 of the Standard Specification field welding of galvanized bearing assemblies, field areas and/or damaged areas of the galvanized coating ASTM A 780.

Include all cost of furnishing and installing steel be components in the lump sum price bid for structural s for structural steel is included in the project. Oth cost in the unit price bid for prestressed beams.

ANCHOR BOLTS

Galvanize all components of anchor bolt assemblies in AASHTO M 232 or ASTM F 2329 as applicable. The weigh assemblies is included in the bent quantities for rei Include all costs of furnishing and installing anchor in the unit price bid for reinforcing steel.

ORIENTATION IN RELATION TO STA Left and right sides, where referred to in these plan to direction of stationing.

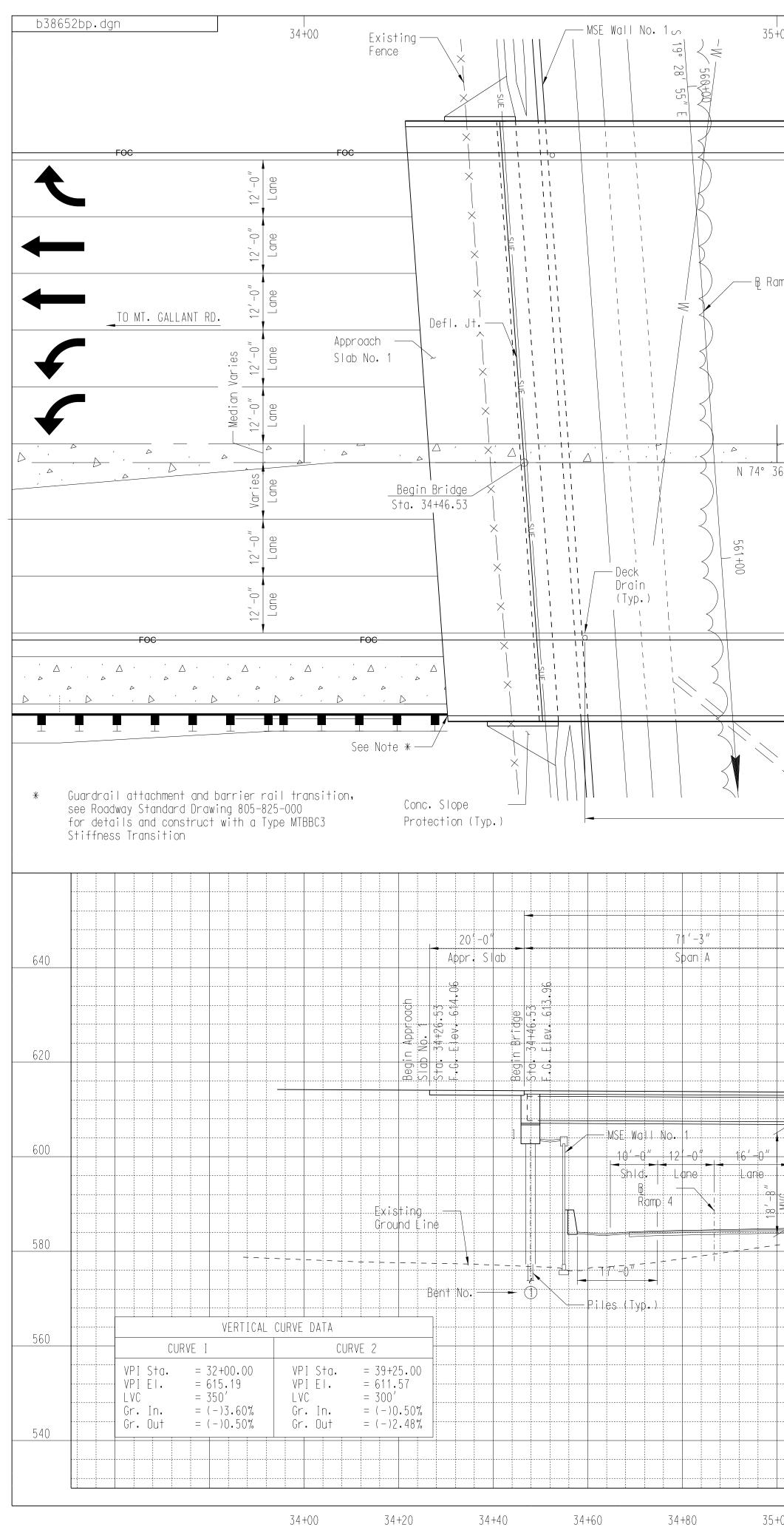
FINAL FINISH OF EXPOSED CONCRETE

Apply the final surface finish on the bridge(s) only checked and designated bridge areas:

- A) Entire surface of all barrier rails, parap approach slab curbs, concrete utility supp and wing walls; outside vertical edge of deck slabs and sidewalks.
- B) Outside face of exterior prestressed girde
- Entire surface of all substructure units, С) except top of bent caps and piers.
- D) MSE Walls: Precast concrete panels and co

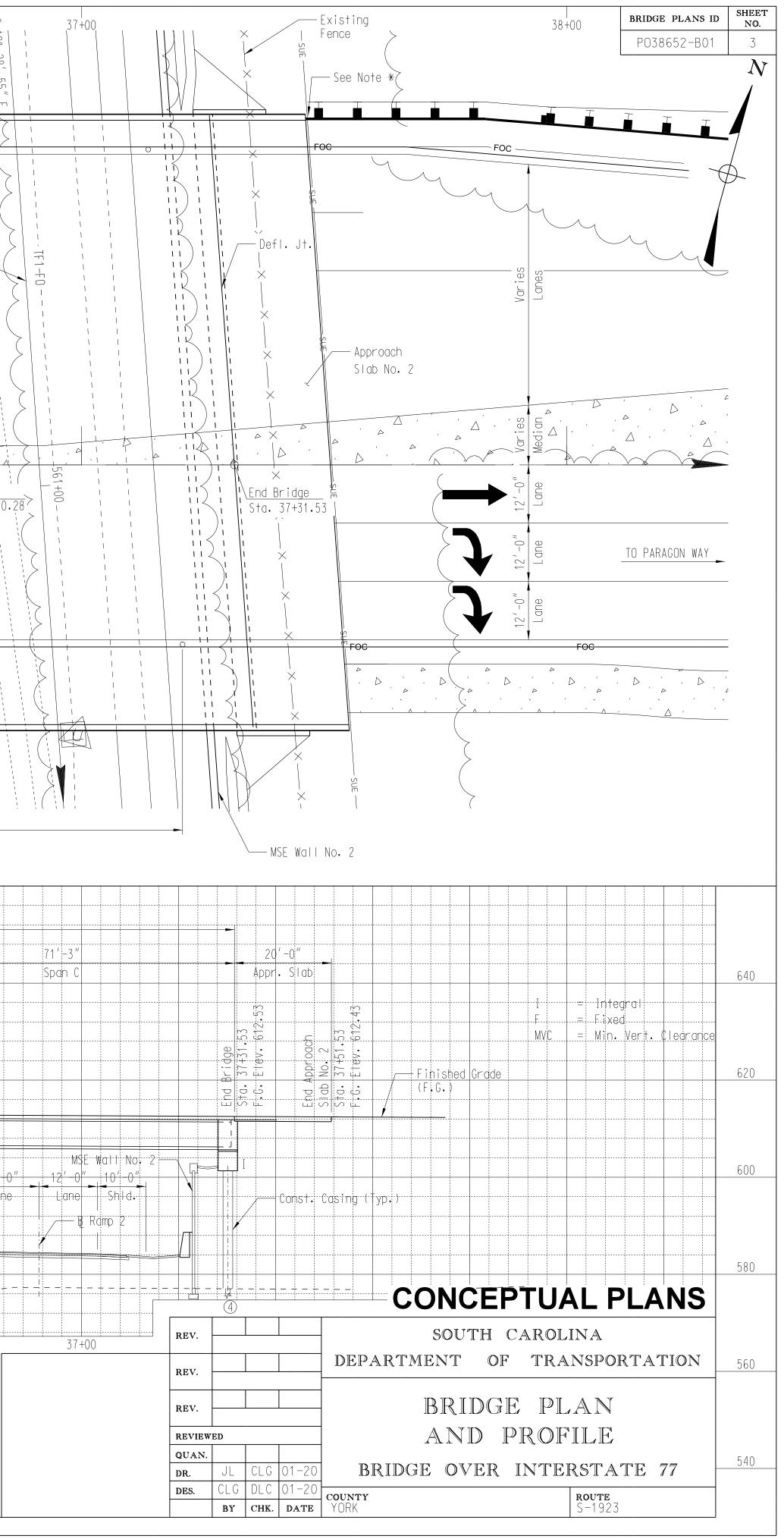
		BRIDGE PLANS ID NO.
	CDEATER A MIANTO	P038652-B01 2
n are horizontal grade,	SPECIFICATIONS AASHTO 2012 LRFD Bridge Design Specificat Interim Revisions through 2013,	ions, 6th Edition, with
rk, install high ed,	ANSI/AASHTO/AWS D1.5 Bridge Welding Code additions and revisions as stated in the	
Fabricator of the given the contract to t up.	DESIGN DATA Load and Resistance Factor Design (LRFD)	Method
tructural steel members.	Live Load: AASHTO HL-93 Loading	
bolts unless	The top I_{4}'' of all concrete slabs is cons and is not included in the slab depth use section properties.	
 However,for than bolt dia, may onnection oversize holes 	All bolted connections, except for steel prestressed concrete beams, are designed having Class "B" contact surfaces.	diaphragm members used with as slip-critical connections
shers are required I a hardened olted connection,	An extra dead load of 0.016 KSF is incorp this structure to accommodate the use of	
rsize and where is made for the	An extra dead load of 0.015 KSF is incorp	
furnishing additional	of this structure as an allowance for a f Seismic Design is in accordance with the Design Specifications for Highway Bridges	2008 SCDOT "Seismic
0 of the Standard	the following parameters: Seismic Design Category: A	
	Analysis Method: No Detailed Analysi	S
or girders, fabricate d paint them using the blies in accordance pplicable.	Operational Classification: II	
assemblies, field e paint in accordance ns, After the required		
d repair the weld in accordance with		
bearing assembly steel if a bid item		
herwise, include the		
n accordance with ht of anchor bolt inforcing steel.		
r bolt assemblies		
ATIONING ns, are in relation		
E SURFACES to the following		
apet walls, pports, bridge		
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coping.		
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	DEV	
	REVIEWED (G.	ENERAL
	QUAN.	NOTES ER INTERSTATE 77
	DES. CLG AWS 03-20 COUNTY	ROUTE
	BY CHK. DATE YORK	S-1923

BRIDGE PLANS ID SHEET

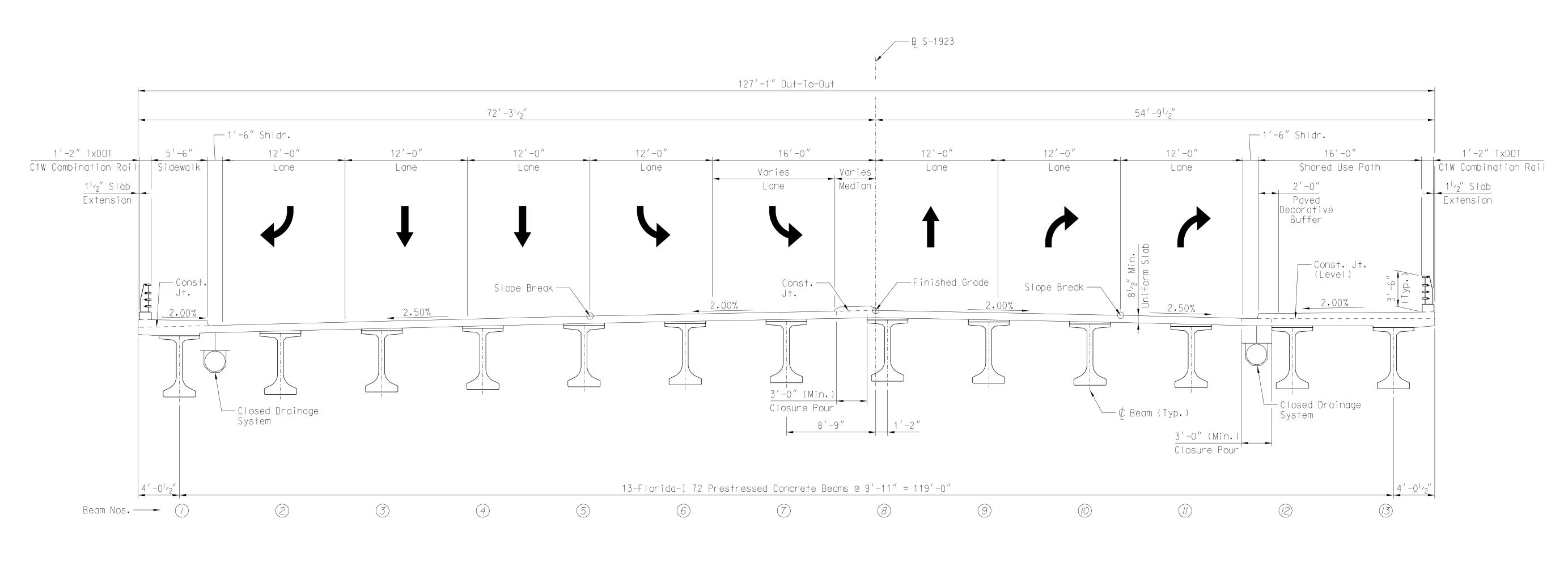


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5+00)							:			1'-2'' TxDNT_C1W	Combination Rail W/ 1 ¹ 2	- - - -	< 19° 28′ 55″ -EE	560+00	36	5+00					— PMVC								S 19° 28' 55" E
amp	- 4 - 40'							, ,	\[\] \[\[\] \[16'-0" 37'-6" Curb To Curb 5 5 61'-6" Curb To Curb	d Use				561+00		05'35'	" (Typ	S-192		TO CHARLOTTE						S	Bent	6+60.
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5+00)			35	+20		35	+40		35	+60		35+	-80		36	5+00			36+2()	36+	40		3	6+60			36	+80







TYPICAL SECTION

(Looking In The Direction Of Stationing)

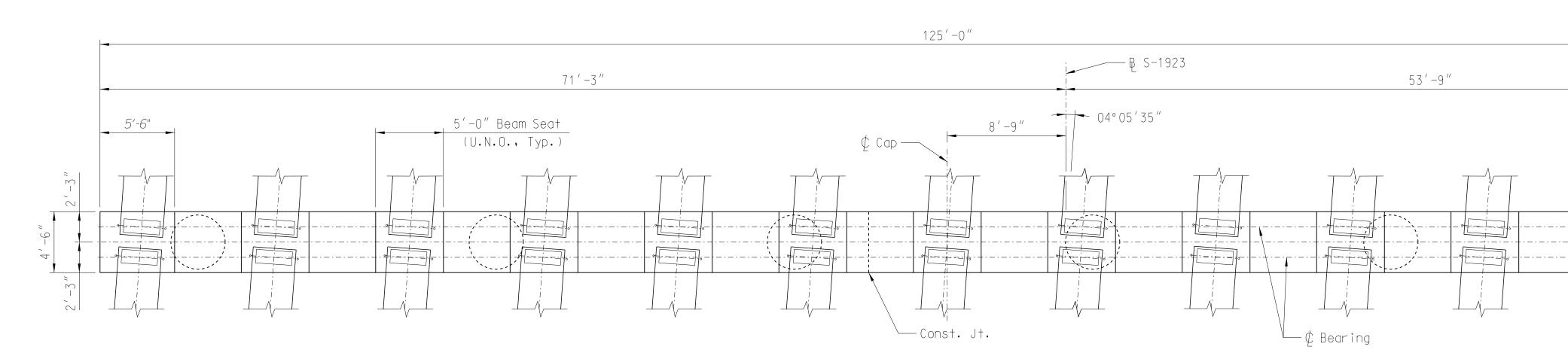
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REV.				SOUTH CAROLINA					
REV.				DEPARTMENT OF TRANSPORTATION					
REV.				TYPICAL SECTION					
REVIEW	VED								
QUAN.									
DR.	JL	CLG	01-20	BRIDGE OVER INTERSTATE 77					
DES.	CLG	DLC	01-20	COUNTY ROUTE					
	BY	CHK.	DATE	YORK S-1923					

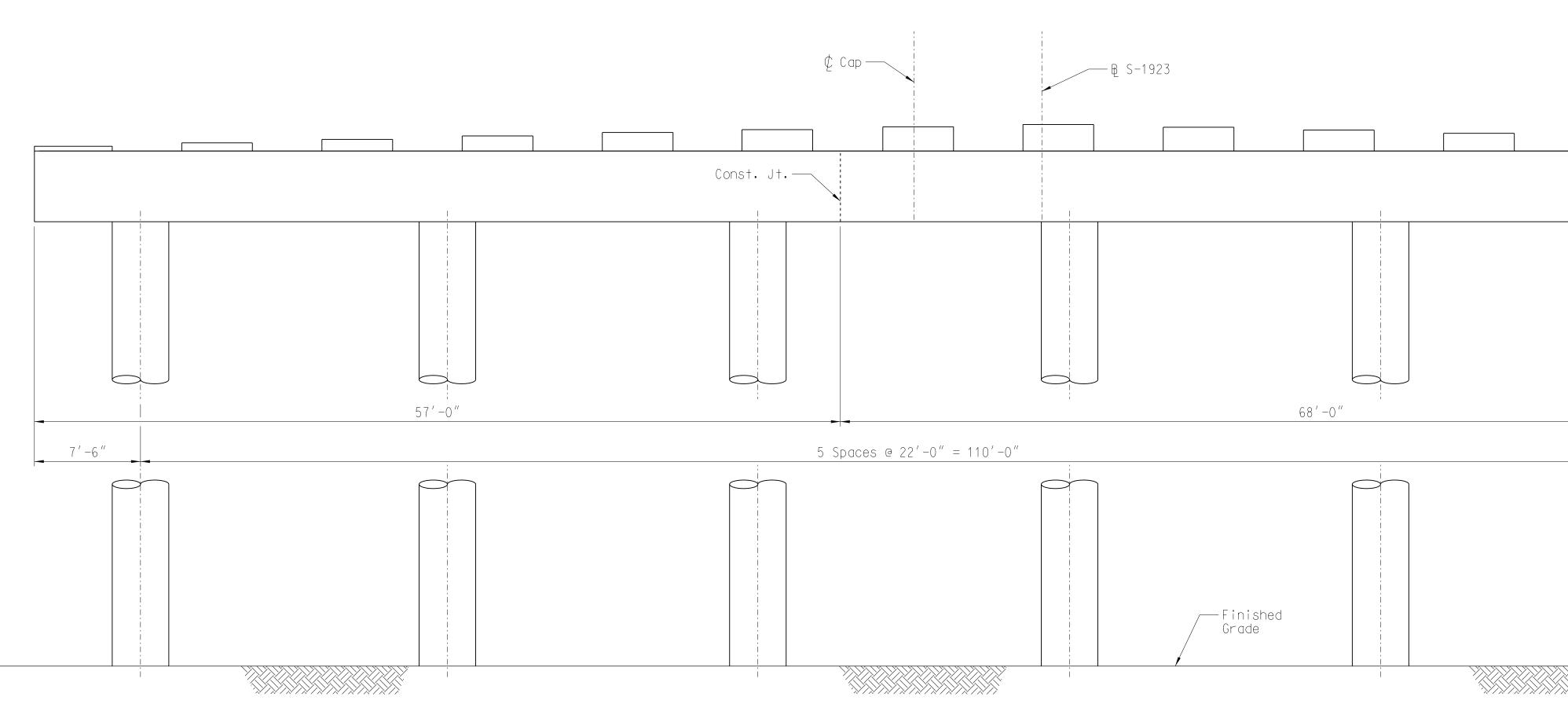




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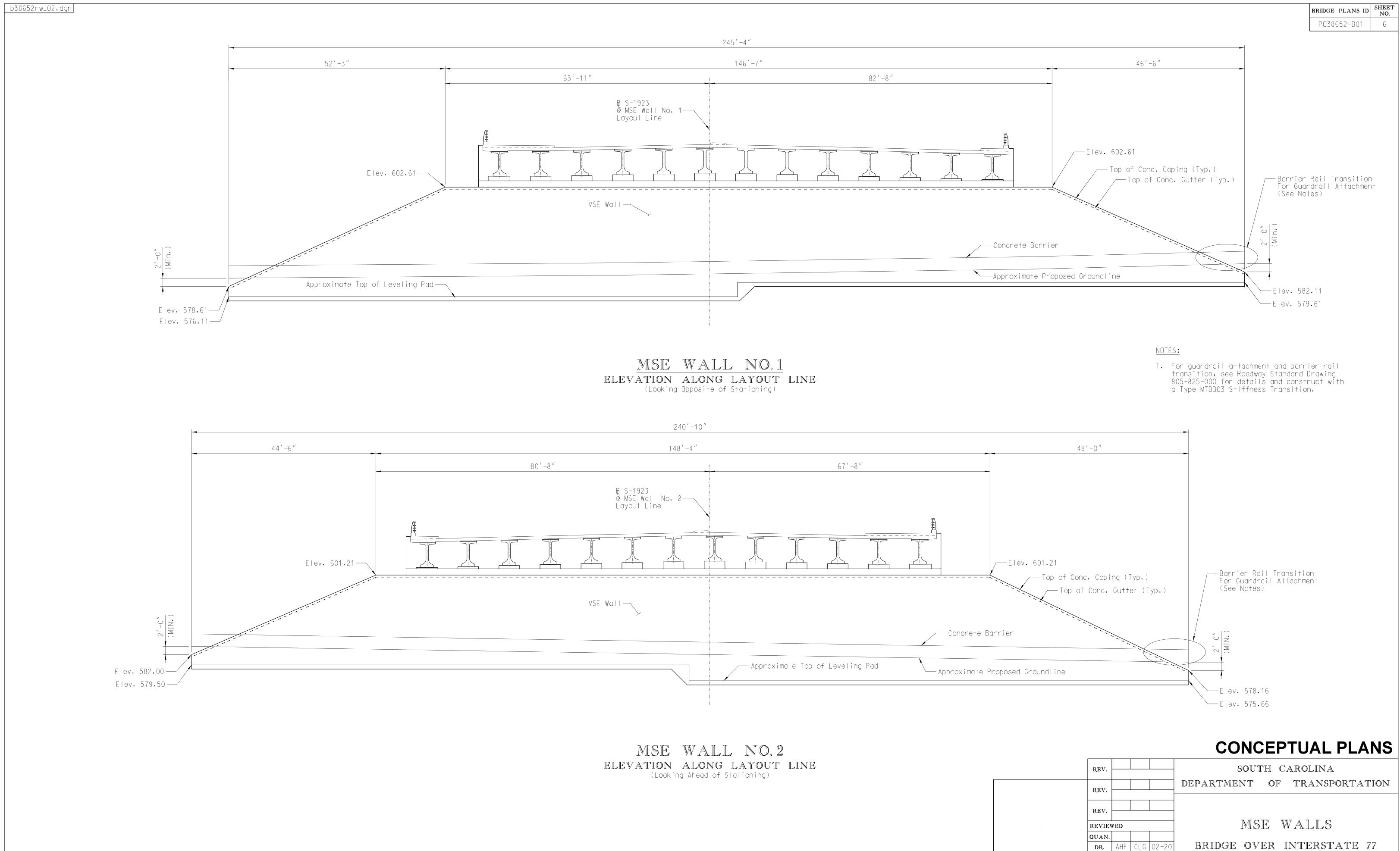
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PLAN

ELEVATION

	NO.
	P038652-B01 5
-	
5′-6″	
¢ Bent	
	2'-3"
	¢ Bent
	<u>4'-0"</u> Dig.
7′-6″	
	Finished Grade
	END ELEVATION
	CONCEPTUAL PLANS
REV.	SOUTH CAROLINA
REV.	DEPARTMENT OF TRANSPORTATION
REV.	INTERIOR BENT
REVIEWED QUAN.	PLAN AND ELEVATION
QUAN. DR. JL CLG 01-20 DES. CLG DLC 01-20	
DES.CLGDLCOT-20BYCHK.DATE	COUNTYROUTEYORKS-1923

BRIDGE PLANS ID SHEET NO.



DES.AHFCLG02-20COUNTYBYCHK.DATEYORK

ROUTE S-1923